

TOWNSHIP OF MILLSTONE
215 MILLSTONE ROAD
PERRINEVILLE NJ 08535
609-208-1920

CERTIFICATE IDENTIFICATION

Date Issued: 03/31/2006
Control #: 12002
Permit #: 20040693

Block: 60.02 Lot: 21.17 Qualification Code:
Work Site Location: 4 HOOPER COURT

MILLSTONE TOWNSHIP

Owner in Fee:

Address: 4 HOOPER COURT

CLARKSBURG NJ 8510

Telephone:

Agent/Contractor: Leon Anthony

Address: 4 HOOPER COURT

CLARKSBURG NJ 8510

Telephone:

Lic. No./ Bldrs. Reg. No.:

Federal Emp. No.:

Social Security No.:

[] CERTIFICATE OF OCCUPANCY

This serves notice that said building or structure has been constructed in accordance with the New Jersey Uniform Construction Code and is approved for occupancy.

[X] CERTIFICATE OF APPROVAL

This serves notice that the work completed has been constructed or installed in accordance with the New Jersey Uniform Construction Code and is approved. If the permit was issued for minor work, this certificate was based upon what was visible at the time of inspection.

[] TEMPORARY CERTIFICATE OF OCCUPANCY/COMPLIANCE

If this is a temporary Certificate of Occupancy or Compliance, the following conditions must be met no later than or will be subject to fine or order to vacate:

HS Salerno

HENRY L. SALERNO Construction Official

U.C.C 260 (rev. 5/03)

1 - APPLICANT 2 - OFFICE 3 - TAX ASSESSOR

Home Warranty No:
Type of Warranty Plan:
Use Group:
Maximum Live Load:
Construction Classification:
Maximum Occupancy Load:
Certificate Exp Date:
Description of Work/Use:
Pole Barn

[] State [] Private
U

Update Desc. of Wk/Use:

[] CERTIFICATE OF CLEARANCE-LEAD ABATEMENT 5:17

This serves notice that based on written certification, lead abatement was performed as per NJAC 5:17, to the following extent:

[] Total removal of lead-based paint hazards in scope of work
[] Partial or limited time period(____ years); see file

[] CERTIFICATE OF CONTINUED OCCUPANCY

This serves notice that based on a general inspection of the visible parts of the building there are no imminent hazards and the building is approved for continued occupancy.

[] CERTIFICATE OF COMPLIANCE

This serves notice that said potentially hazardous equipment has been installed and/or maintained in accordance with the New Jersey Uniform Construction Code and is approved for use until

Fees: \$0.00

Paid[X] Check No.: 5332

Collected by: LM





**BUILDING
SUBCODE
TECHNICAL SECTION**

A. IDENTIFICATION—APPLICANT: COMPLETE ALL APPLICABLE INFORMATION. WHEN CHANGING CONTRACTORS, NOTIFY THIS OFFICE. CALL UTILITY DIG NO. 1-800-272-1000.

Block 60.02 Lot 21.17
Work Site Location 41 Hopper Ct
Owner in Fee [REDACTED]
Address 4 Hopper Ct
Tele. [REDACTED]
Contractor [REDACTED]
Address 1170
Tele. [REDACTED] Fax [REDACTED]
Lic. No. or Bids. Reg. No. [REDACTED]
Federal Emp. No. [REDACTED]

JOB SUMMARY (Office Use Only)

PLAN REVIEW		INSPECTIONS		Dates (Month/Day)	
Initial	Date	Initial	Date	Failure	Approval
☐ All	☒ No-Plans Required	Type:			
☐ Footing	☐ Footing				
☐ Foundation	☐ Foundation				
☐ Frame	☐ Frame				
☐ Other	☐ Other				
Joint Plan Review Required:					
☐ Elec.	☐ Plumb.	☐ Fire	☐ Elevator		
SUBCODE APPROVAL					
☐ CO	☐ CCG	☐ CA			
Date:	<u>3-31-06</u>				
Approved by:	<u>[Signature]</u>				
Final	<u>12/20/05</u>				
Barrier-Free	<u>2/26/06</u>				

B. BUILDING CHARACTERISTICS

Use Group Present _____ Proposed _____
Constr. Class Present _____ Proposed _____
No. of Stories 1
Height of Structure _____ Ft.
Area — Largest Floor 884 Sq. Ft.
New Bldg. Area All Floors 484 Sq. Ft.
Volume of New Structure 4492 Cu. Ft.
Total Land Area Disturbed 884 Sq. Ft.

Est. Cost of Bldg. Work:
1. New Bldg. \$ 5600
2. Alteration \$ 5600
3. Total (1+2) \$ 11200



Date Received _____
Date Issued _____
Control # _____
Permit # _____

C. CERTIFICATION IN LIEU OF OATH

I hereby certify that I am the (agent of) owner of record and am authorized to make this application.

Signature _____

D. TECHNICAL SITE DATA

DESCRIPTION OF WORK pole barn
26x34
13' high with vinyl siding
w/ 4' m. base
13' high with vinyl siding
not steel siding

TYPE OF WORK:

- ☐ New Building
- ☒ Addition Barn (pole)
- ☐ Alteration
- ☐ Roofing
- ☐ Siding
- ☐ Fence
- ☐ Sign
- ☐ Pool
- ☐ Asbestos Abatement Subchapter 8
- ☐ Lead Haz. Abatement NJAC 5.17
- ☐ Other
- ☐ Demolition

FEE (Office Use Only)
\$ 165

Administrative Surcharge \$ _____
Minimum Fee \$ _____
DCA Training Fee \$ 31
TOTAL FEE \$ _____

Millstone Township Construction Department
470 Stagecoach Road
Clarksburg, New Jersey
Phone 609-208-1920 , Fax 609-208-1746

I UNDERSTAND THAT WHEN A PERMIT IS ISSUED FOR:

Address: 4 Hooper Ct Clarksburg
Block: 60.02 Lot: 21.17
Type of work: Pole Barn

I will be responsible for requesting all required inspections for work permitted at the above address in accordance with N.J.A.C. 5:23-2.18.

I further understand that failure to call for all required inspections will make me liable for a monetary penalty in accordance with N.J.A.C. 5:23-2.31.

Failure to notify this office of the completion of the above mentioned project will subject you to a monetary penalty in accordance with N.J.A.C. 5:23-2.31.

Name: [Redacted] / [Redacted]
Please print name of property owner or responsible person in charge of work Company responsible for work

Address: 4 Hooper Ct Clarksburg NJ

Phone: [Redacted] Fax: [Redacted]

Signature: [Redacted] Date: 9/20/04

If construction does not commence within one (1) year of date of issuance or if construction ceases for a period of six (6) months, construction permit is void.

In accordance with New Jersey State Law 13:45A-16.2

**FINAL INSPECTIONS ARE REQUIRED
BEFORE FINAL PAYMENT
IS MADE TO CONTRACTOR**

12002-

Application for Zoning Permit

Fee \$50.00

Township of Millstone

Residential/ Commercial

Block: 60.02 Lot: 21.17 Zone: _____

Work site location: 4 Hooper Ct Clarksburg

Property owner: [REDACTED]

Address of owner: 4 Hooper Ct Clarksburg

Existing use: _____ Proposed use: _____ Lot Coverage: _____

Description of work: 26 x 34 Pole Barn

Submit one copy of survey/plot plan indicating size of proposed construction
Three copies of survey for new single-family dwelling

Setbacks:

Front: _____ Rear: 30 Left Sides: _____ Right Side: 25 Height: _____

Size of Lot (acreage): _____ Corner lot: _____ Inside lot: ✓

Survey/Plot plan must indicate any wetlands, conservation, drainage and utilities easements. I hereby certify that the proposed information above is true, and agree to conform to all applicable laws of this jurisdiction.

[REDACTED]
Signature/applicant

[REDACTED]
telephone

[REDACTED]
fax

FOR OFFICE USE ONLY

Denial: _____
Signature Zoning Officer _____ date _____

Comments: _____

Approval: [Signature]
Signature Zoning Officer _____ date 11/12/04

Date paid 11/12/04 Check # 5333

Received by [Signature]

Kevin C. Roy, Architect

34 MAIN ST. , P.O. BOX 777, ENGLISHTOWN, N.J.
732-620-8642 732-446-6400 FAX

October 12, 2004

To: Bldg Code Official
Millstone Twp., NJ

Re: POLE BARN
Block 60.02 Lot 21.17
[REDACTED]
4 Hooper Court
Millstone Twp., NJ

FILE COPY

BUILDING PLANS
RELEASED *REASON*
BY *[Signature]*

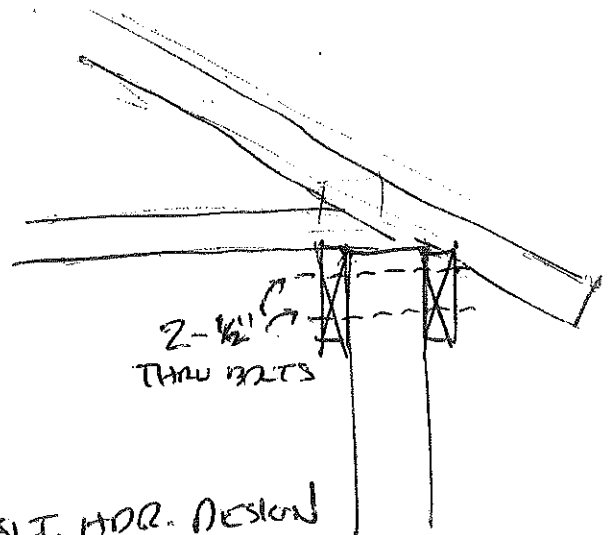
The above client plans to build a 26' x 34' wood post structure at the above location for use as a horse stable. This office has reviewed the plans designed by the L.F. Garlinghouse Co. Inc. submitted under the header of 84 Lumber.

The structure consists of Manufacturer designed wood trusses supported on a double 2x12 perimeter beam at the 10'-0" level above grade. Connect the trusses to the support beam with galvanized hurricane style clips. The double 2x12 beam is connected to 6x6 treated wood columns, fourteen columns are positioned along the perimeter at approximately 8'-0" spacing. The 2x12 are fastened one on each side of each column and thru bolted with two 1/2" dia carriage bolts. The 6x6 posts extend to below grade and shall rest on 14"x14" concrete footings of 12" thickness. The footing bottom shall be at least 36" below grade.

Based on these observations I certify that these structural elements are sufficient to support the new building and therefore satisfy the intent of the Building Code and meet with the approval of this Architect.

Kevin Roy
Architect

[Handwritten Signature]



FILE NOTE

BLOCK _____ LOT _____ NAME _____

Inspector/Secretary Informal Notes:

Date: 9/23/04 Comments: Plans To be sealed by
AIA. MESSAGE

Date: _____ Comments: [REDACTED] picked up plans
9/28/04

Date: 10/15/04 Comments: sealed plans
called MRE-

Date: _____ Comments: _____

Date: _____ Comments: _____

Date: _____ Comments: _____

Block 60.02 Lot 21.17 Name _____

**The Township of Millstone
Construction Department
County of Monmouth
State of New Jersey**

Henry L. Salerno, Construction Official
Phone 609-208-1920
Fax 609-208-1746

P.O. Box 240
215 Millstone Road
Perrineville, N.J. 08535

Dear Homeowner:

Date: 2/1/06

It has come to the attention of this department that the following condition exists:

BLOCK 60.02 LOT 21.17 PERMIT # 04 693
ADDRESS: 4 Hooper Ct TYPE Pole Barn

- ☒ Open permit. Project is not completed. Please finalize and call for inspections.
- ☒ Permit is about to expire. Please contact this office for an extension before you need to repay/reapply for the permit.
- ☐ Working without a permit. Must stop work and file immediately.
- ☐ Structure installed without benefit of a permit. Please call and file proper paperwork.
- ☒ Never called for inspections. Please contact this office to set up inspections within the next five (5) days.
- ☐ Never picked up/paid for permit. Please remit \$ _____
- ☐ Need update. Work has exceeded original permit. Please file appropriate tech cards.
- ☐ Need update for change of contractor.

FAILURE TO ACT ON THIS NOTICE COULD RESULT IN FORMAL NOTICE OF VIOLATION BEING FILED.

The owner and agent contractor bear joint responsibility for bringing about compliance. Should you need more information contact us.

INSPECTORS OFFICE HOURS:

Building (Mon-Fri 8:30 AM- 10 AM & 3PM-4 PM)

Electric (Mon-Fri 8:30AM- 9:30AM)

Fire (Mon-Fri 8:30AM-10 AM & 3PM- 4PM) **Plumbing** (Mon-Wed-Fri 8:30 AM- 10 AM & Mon-Fri 2PM-4PM)

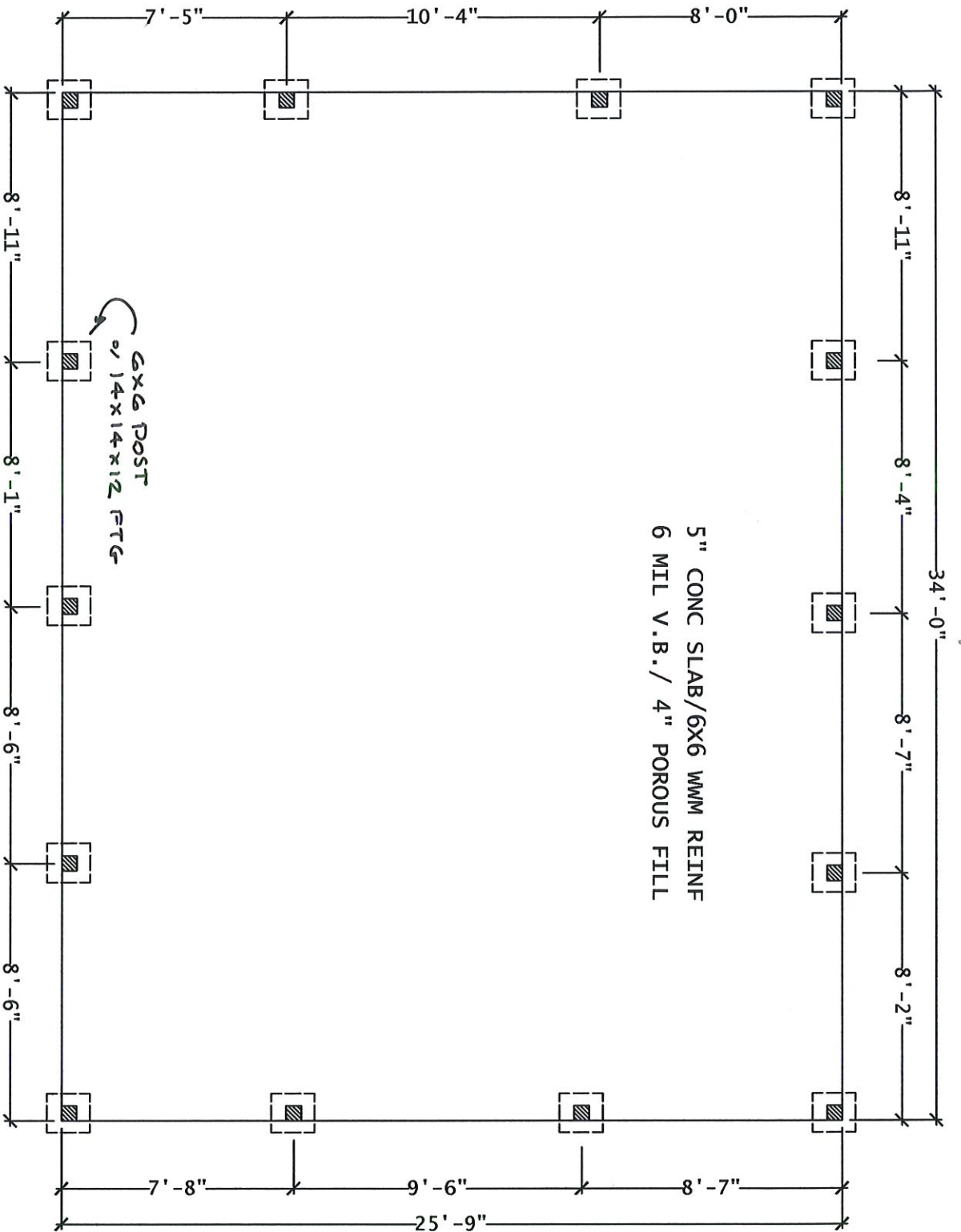
NEEDED INSPECTIONS:

Building
Electric
Fire
Plumbing

Sincerely,

Valer Wankler

Technical Assistant to the Construction Official



NOTES:

TOTAL FLOOR LOAD 100 PSF
TOTAL ROOF LOAD 35 PSF

TYPE 5B CONST

USR GROUP U

CODE: IBC 2000 NJ ED

CONC: 3000 PSI

FOUNDATION PLAN NTS

UTILITY BLDG

Block 60.02 Lot 21.17

4 HOOPER CT., MILLSTONE TWP, NJ

Kevin C Roy, Architect

34 Main St., PO Box 777, Englishtown, NJ

732-620-8642 732-446-6400 fax

LICENSES & CERTIFICATIONS

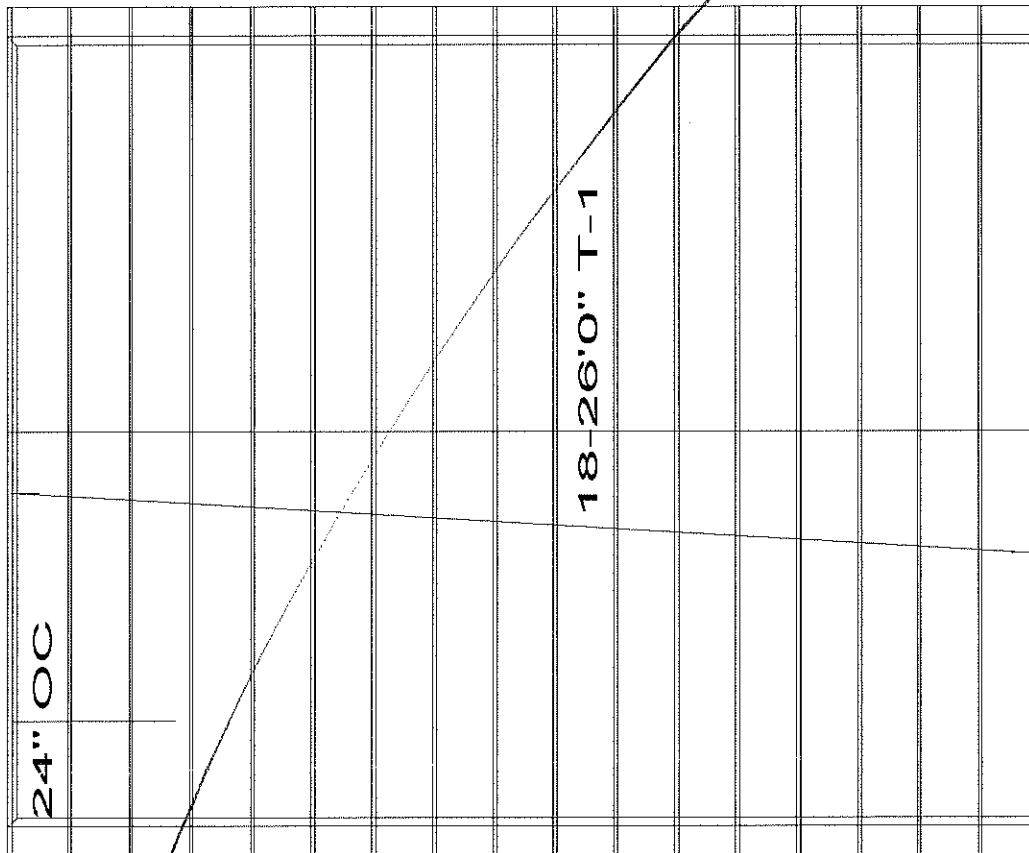
NEW JERSEY RA14415

OHIO RA9537

January 10, 2005

FILE COPY

26'0" X 34'0" POLE BARN
4 HOOPER COURT - MILLSTONE
ROOF TRUSS LAYOUT
REF:L1/04S140 (9-30-2004)



LAYOUT FOR
TRUSS PLACEMENT ONLY

TRUSS SYSTEMS INC.
P.O. BOX 233
ADELPHIA, NJ 07710-0233

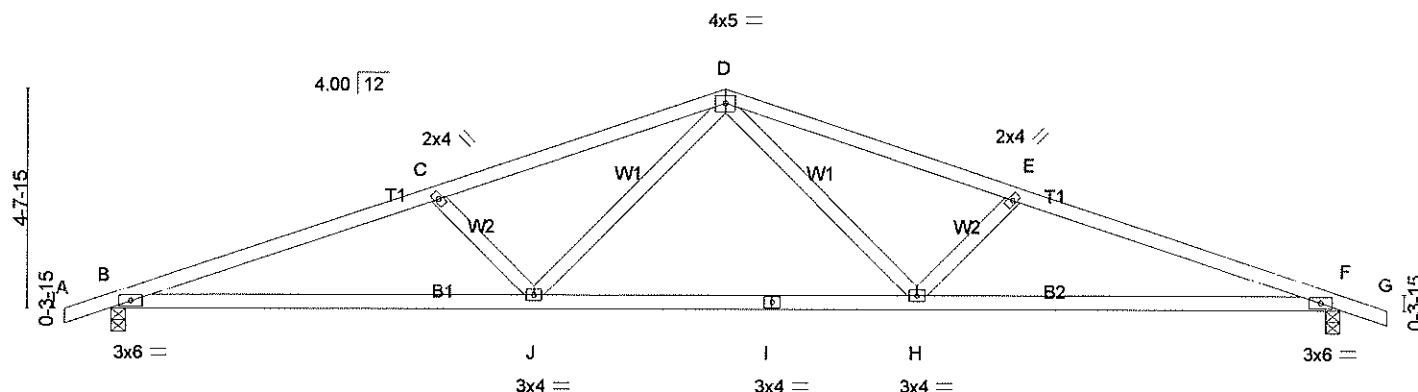
Job	Truss	Truss Type	Qty	Ply	4 HOOPER COURT
04S140	T-1	FINK	18	1	Job Reference (optional)

Truss Systems Incorporated, Adelphia, NJ. 07710

5.200 s Aug 19 2003 MiTek Industries, Inc. Thu Sep 30 09:36:16 2004 Page 1

-1-0-0	6-10-14	13-0-0	19-1-2	26-0-0	27-0-0
1-0-0	6-10-14	6-1-2	6-1-2	6-10-14	1-0-0

Scale = 1:48.8



8-11-4	17-0-12	26-0-0
8-11-4	8-1-8	8-11-4

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.8	Plates Increase	1.15	TC 0.85	Vert(LL)	-0.40	H-J	>766	360	MII20	249/190
TCDL 7.0	Lumber Increase	1.15	BC 0.90	Vert(TL)	-0.59	H-J	>526	240		
BCLL 20.0	Rep Stress Incr	NO	WB 0.42	Horz(TL)	0.08	F	n/a	n/a		
BCDL 10.0	Code IRC2000/ANSI95		(Simplified)							
									Weight: 109 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1
BOT CHORD 2 X 4 SYP DSS
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Sheathed or 2-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-4-5 oc bracing.

REACTIONS (lb/size) B=1549/0-3-8, F=1549/0-3-8

Max Horz B=148(load case 8)
Max Uplift B=-970(load case 6), F=-970(load case 7)
Max Grav B=1715(load case 2), F=1715(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD A-B=0/14, B-C=-3308/2700, C-D=-2927/2426, D-E=-2927/2426, E-F=-3308/2700, F-G=0/14
BOT CHORD B-J=-2394/3127, I-J=-1480/2076, H-I=-1480/2076, F-H=-2394/3127
WEBS C-J=-521/705, D-J=-634/1213, D-H=-634/1213, E-H=-521/705

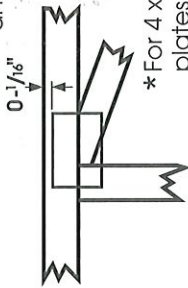
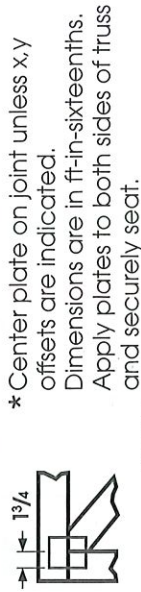
NOTES

- 1) Wind: ASCE 7-98; 115mph; h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) TCLL: ASCE 7-98; Pg= 30.0 psf (ground snow); Category II; Exp C; Fully Exp.; Ct=1.1
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table 1607.1 of IBC-00.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 970 lb uplift at joint B and 970 lb uplift at joint F.

LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

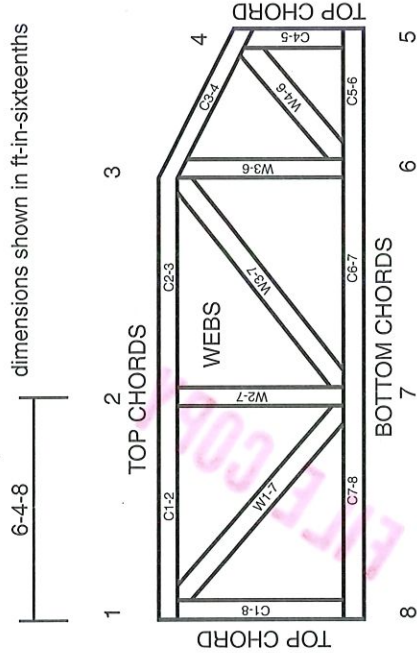


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

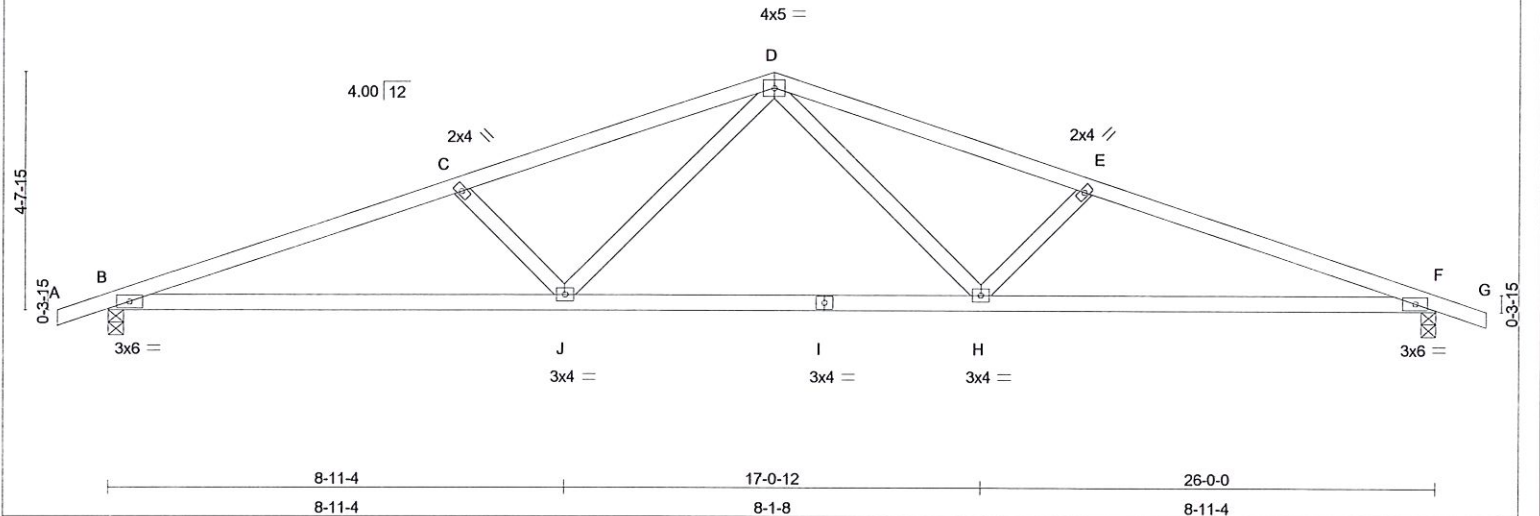


General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Install and load vertically unless indicated otherwise.

Job 04S140	Truss T-1	Truss Type FINK	Qty 18	Ply 1	4 HOOPER COURT	17311315
Truss Systems Incorporated, Adelphia, NJ. 07710			Job Reference (optional) 5.200 s Jan 16 2004 MiTek Industries, Inc. Mon Oct 11 14:46:43 2004 Page 1			



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.8	2-0-0	TC 0.85	in (loc) l/def L/d	MII20	249/190
TCDL 7.0	Plates Increase 1.15	BC 0.90	Vert(LL) -0.40 H-J >766 360		
BCLL 20.0	Lumber Increase 1.15	WB 0.42	Vert(TL) -0.59 H-J >526 240		
BCDL 10.0	Rep Stress Incr NO	(Simplified)	Horz(TL) 0.08 F n/a n/a		
	Code IRC2000/ANSI95				
				Weight: 109 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.1	TOP CHORD Sheathed or 2-7-2 oc purlins.
BOT CHORD 2 X 4 SYP DSS	BOT CHORD Rigid ceiling directly applied or 4-4-5 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) B=1549/0-3-8, F=1549/0-3-8
 Max Horz B=148(load case 8)
 Max Uplift B=-970(load case 6), F=-970(load case 7)
 Max Grav B=1715(load case 2), F=1715(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/14, B-C=-3308/2700, C-D=-2927/2426, D-E=-2927/2426, E-F=-3308/2700, F-G=0/14
 BOT CHORD B-J=-2394/3127, I-J=-1480/2076, H-I=-1480/2076, F-H=-2394/3127
 WEBS C-J=-521/705, D-J=-634/1213, D-H=-634/1213, E-H=-521/705

NOTES

- 1) Wind: ASCE 7-98; 115mph; h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) TCLL: ASCE 7-98; Pg= 30.0 psf (ground snow); Category II; Exp C; Fully Exp.; Ct=1.1
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table 1607.1 of IBC-00.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 970 lb uplift at joint B and 970 lb uplift at joint F.

LOAD CASE(S) Standard

FILE COPY

John Salter

October 12, 2004

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

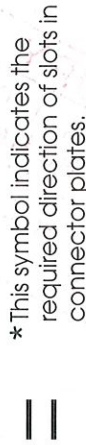
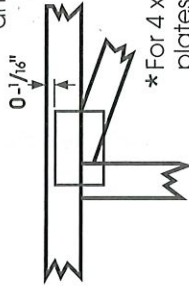
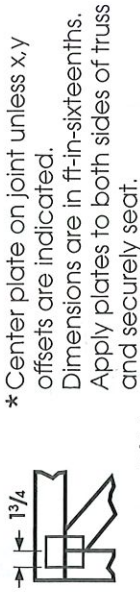
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty,
Suite #300
Chesterfield, MO 63017



Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



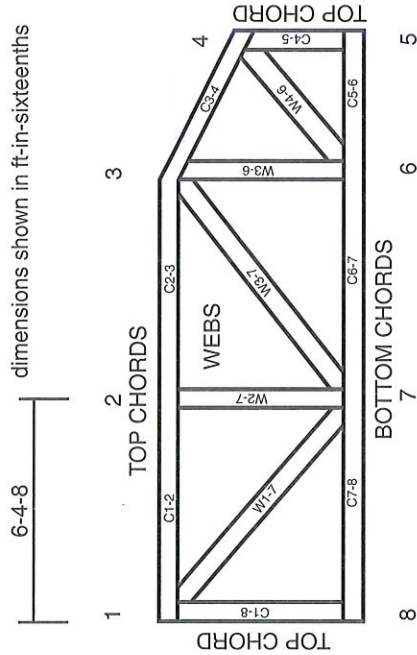
BEARING



Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

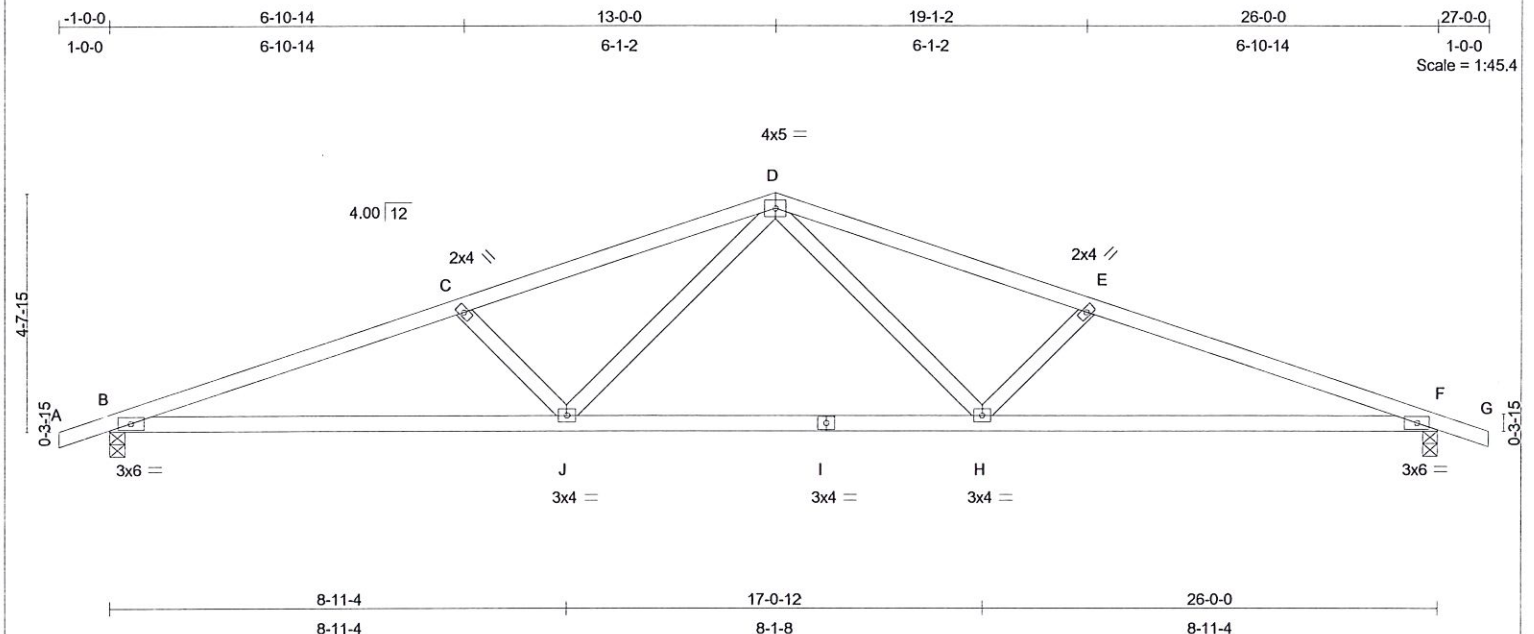


General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Install and load vertically unless indicated otherwise.

Job 04S140	Truss T-1	Truss Type FINK	Qty 18	Ply 1	4 HOOPER COURT	17311315
Truss Systems Incorporated, Adelphia, NJ. 07710			Job Reference (optional) 5.200 s Jan 16 2004 MiTek Industries, Inc. Mon Oct 11 14:46:43 2004 Page 1			



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.8	2-0-0	TC 0.85	in (loc) l/defl L/d	MII20	249/190
TCDL 7.0	Plates Increase 1.15	BC 0.90	Vert(LL) -0.40 H-J >766 360		
BCLL 20.0	Lumber Increase 1.15	WB 0.42	Vert(TL) -0.59 H-J >526 240		
BCDL 10.0	Rep Stress Incr NO	(Simplified)	Horz(TL) 0.08 F n/a n/a		
	Code IRC2000/ANSI95			Weight: 109 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.1	TOP CHORD Sheathed or 2-7-2 oc purlins.
BOT CHORD 2 X 4 SYP DSS	BOT CHORD Rigid ceiling directly applied or 4-4-5 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) B=1549/0-3-8, F=1549/0-3-8
 Max Horz B=148(load case 8)
 Max Uplift B=-970(load case 6), F=-970(load case 7)
 Max Grav B=1715(load case 2), F=1715(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=0/14, B-C=-3308/2700, C-D=-2927/2426, D-E=-2927/2426, E-F=-3308/2700, F-G=0/14
 BOT CHORD B-J=-2394/3127, I-J=-1480/2076, H-I=-1480/2076, F-H=-2394/3127
 WEBS C-J=-521/705, D-J=-634/1213, D-H=-634/1213, E-H=-521/705

- NOTES**
- 1) Wind: ASCE 7-98; 115mph; h=25ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; partially; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) TCLL: ASCE 7-98; Pg= 30.0 psf (ground snow); Category II; Exp C; Fully Exp.; Ct=1.1
 - 3) Unbalanced snow loads have been considered for this design.
 - 4) This truss has been designed for 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table 1607.1 of IBC-00.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 970 lb uplift at joint B and 970 lb uplift at joint F.

LOAD CASE(S) Standard

FIELD COPY
 Note: This Set of Plans Must Be Kept On Job Site At All Times & For Every Inspection

John D. Smith
 October 12, 2004

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty,
 Suite #300
 Chesterfield, MO 63017

